APPENDIX A: STANDARD DETAIL DRAWINGS

Streets

Figure S-1: General Notes—Paving

GENERAL NOTES

- 1. All costs associated with testing shall be the responsibility of the developer/contractor.
- 2. Unless otherwise required the following testing schedule for earthwork and subgrade testing shall be followed:

A. B.	Soil classification - Standard proctor -	1200 sq. Yds. of material or as necessary to determine uniformity of material. as necessary to provide information for required densities.
C.	Subgrade density -	a maximum of every 1200 sq. yds. of surface area or as necessary to determine uniformity of compaction.
D.	Embankment density -	each 8" layer at a maximum of every 1200 sq. yds. of surface area, or as necessary to determine uniformity of compaction.
E.	Trench under paving -	one (1) subgrade density for every 200 LF, of trench or any distinct transverse crossing.
F. G.	California bearing ratio - Paving Core -	as deemed necessary by the city to ensure subgrade meets specifications. A minimum of three (3), or one (1) for each 6,000 sq. yds. of paving, whichever is greater, to determine paving thickness.

- Advance notification (2 hours minimum) shall be required prior to the taking of any density test. Notification shall be made to the Street Supt. City shall choose testing locations.
- Unless otherwise shown, a minimum of 95% of standard proctor density @ ±2% of optimum moisture is required
 for each density taken. Two (2) additional tests shall be performed for each failed test on transverse crossings under
 paving.
- Any device used for supporting dowels shall have sufficient rigidity and be so held in place during concrete
 placement that dowels will be in true position in the finished pavement. Any device not producing the desired results
 shall be rejected.
- Cold-drawn steel wire for concrete reinforcement meeting the requirements of AASHTO M32 shall be used for all baskets, spacers, and stakes.
- Dowel bars shall conform to material requirements of AASHTO Section M31, M42 or M53; grade 60. Dowel bars shall be centered on the basket regardless of the width of the basket or the length of the dowel bar.
- The height of the load transfer unit (measured to the center of the dowel bar) shall be 1/2 the thickness of the
 pavement.
- 9. Dowel bars shall have a shop or field coat of lead or zinc chromate primer for full length of bars, and alternate ends shall have a field coat of mc-70 (or other heavy lubricant adequate for breaking the bond between the steel and concrete) for a minimum of 1/2 the length of the bars.
- 10. The lubricated ends of the dowel bars shall have expansion caps with a minimum 1" and a maximum 2" air space in the end of the expansion caps (expansion joint assemblies).
- 11. City may require proof rolling with a loaded truck min. 50,000 lbs to ensure stability of base.
- Joint sealer shall be ASTM 3405 for paving or ASTM C-920, Type M, Grade NS, Class 25 for vertical face of curb joints.
- Methods and equipment used for joint sealing shall be in accordance with the latest edition of the ODOT Standard Specifications.

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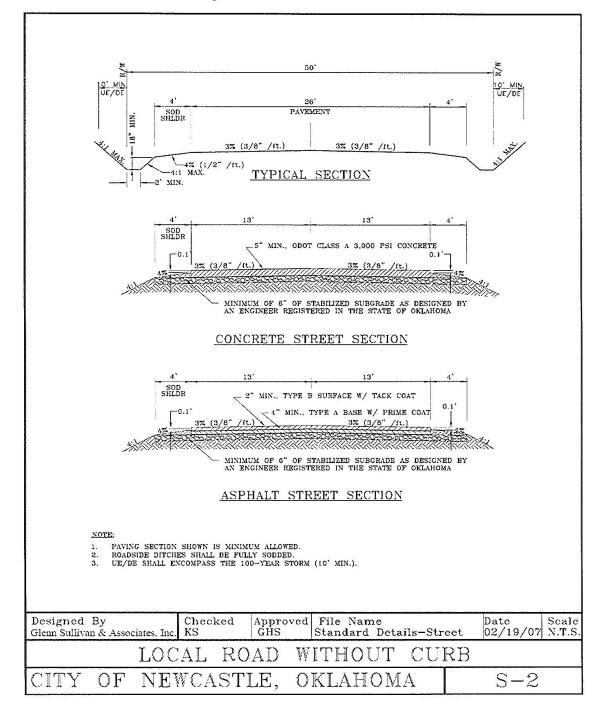


Figure S-2: Local Road Without Curb

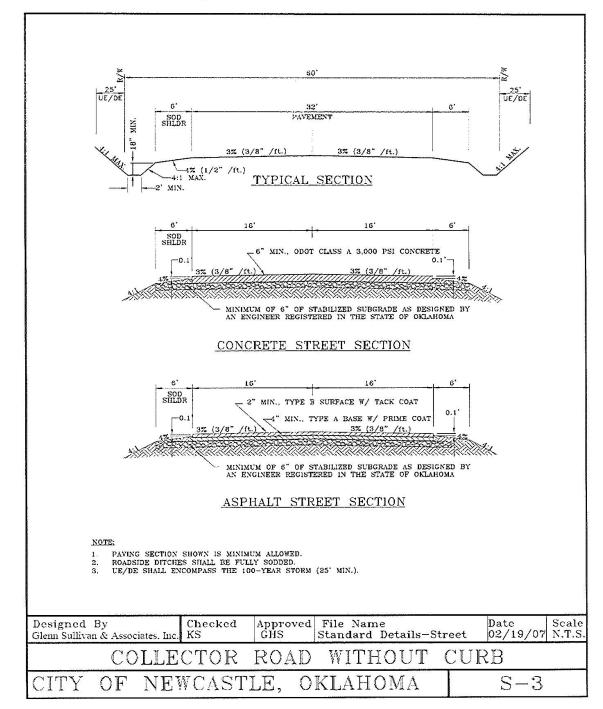
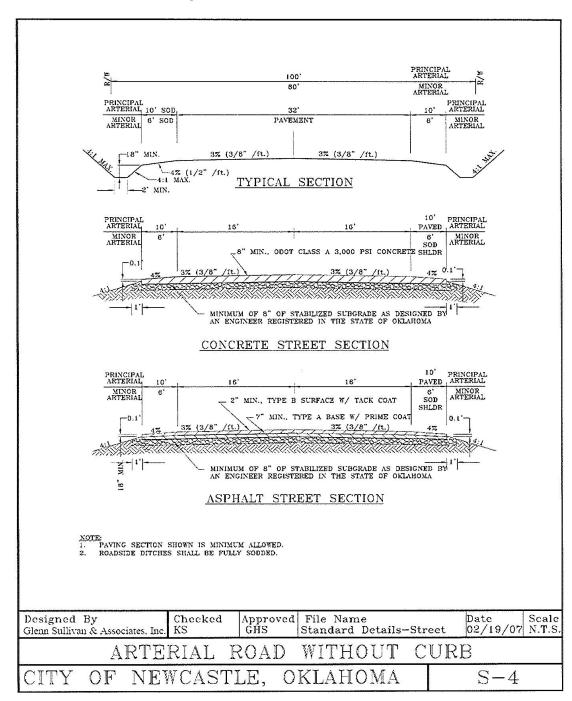


Figure S-3: Collector Road Without Curb

Figure S-4: Arterial Road Without Curb



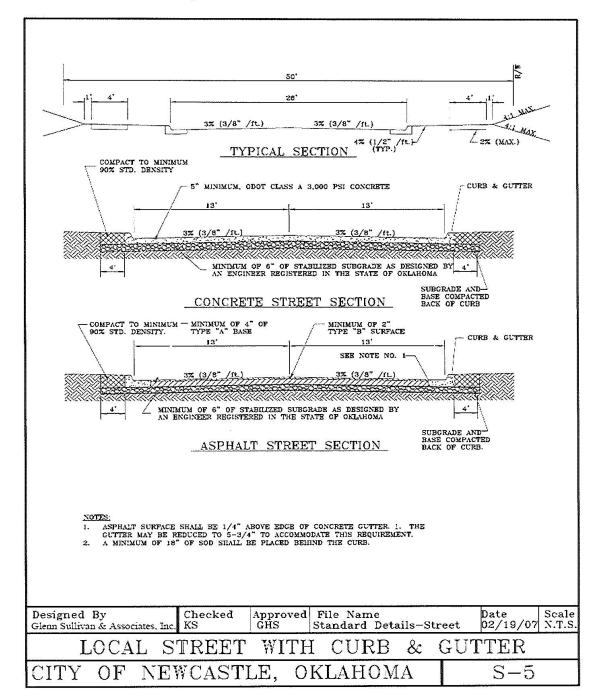
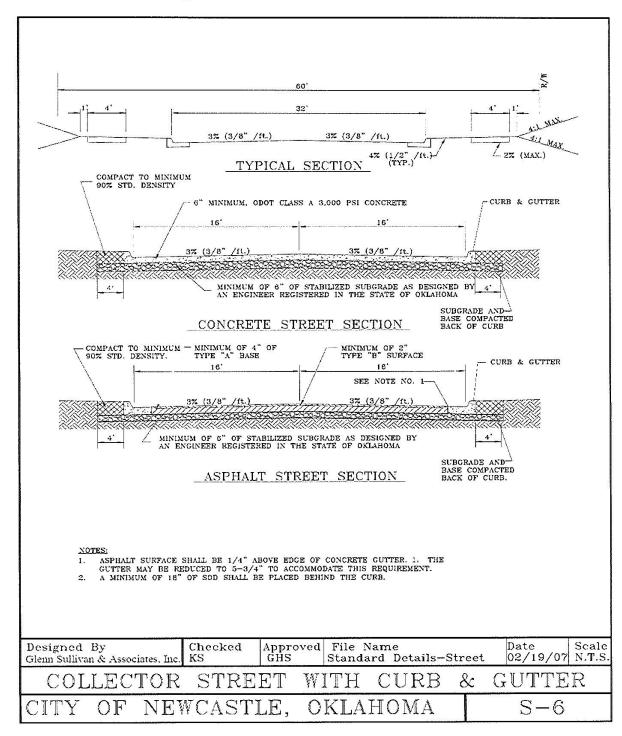


Figure S-5: Local Street With Curb & Gutter

Figure S-6: Collector Street With Curb & Gutter



PRINCIPAL ARTERIAL BO, MINOR 32 3% (3/8" /ft.) 3% (3/8" /ft.) ALL_MAX. ∠ 2% (MAX.) TYPICAL SECTION COMPACT TO MINIMUM 90% STD. DENSITY 8" MINIMUM, ODOT CLASS A 3,000 PSI CONCRETE - CURB & GUTTER 3% (3/8" /ft.) 3% (3/8" /11.) MINIMUM OF 8" OF STABILIZED SUBGRADE AS DESIGNED BY 4. AN ENGINEER REGISTERED IN THE STATE OF OKLAHOMA SUBGRADE AND-BASE COMPACTED BACK OF CURB CONCRETE STREET SECTION MINIMUM OF 6" OF TYPE "A" BASE MINIMUM OF 2" TYPE "B" SURFACE CURB & GUTTER 16' 16' SEE NOTE NO. 1-MINIMUM OF 8" OF STABILIZED SUBGRADE AS DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF OXLAHOMA SUBGRADE AND BASE COMPACTED BACK OF CURB. ASPHALT STREET SECTION NOTES: ASPHALT SURFACE SHALL BE 1/4" ABOVE EDGE OF CONCRETE GUTTER. I. THE GUTTER MAY BE REDUCED TO 5-3/4" TO ACCOMMODATE THIS REQUIREMENT. A MINIMUM OF 18" OF SOD SHALL BE PLACED BEHIND THE CURB. Approved GHS Designed By Checked File Name Date Scale 02/19/07 N.T.S Glenn Sullivan & Associates, Inc Standard Details-Street ARTERIAL STREET WITH CURB & GUTTER OKLAHOMA NEWCASTLE,

Figure S-7: Arterial Street With Curb & Gutter

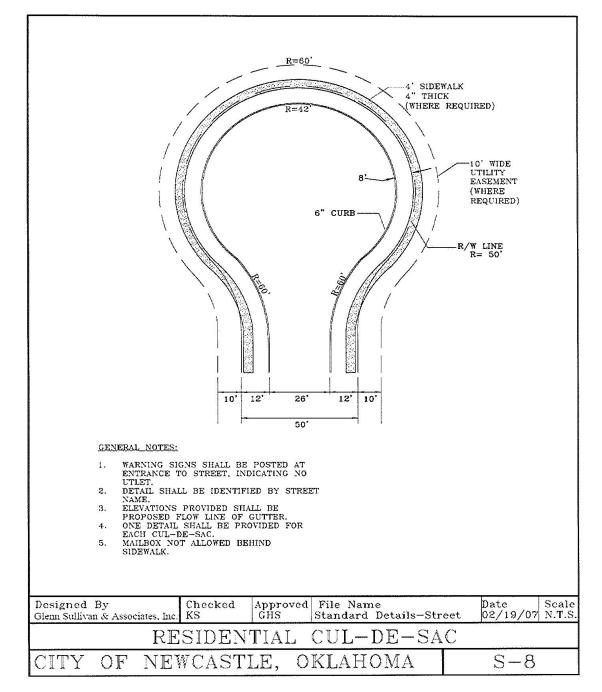


Figure S-8: Residential Cul-de-Sac

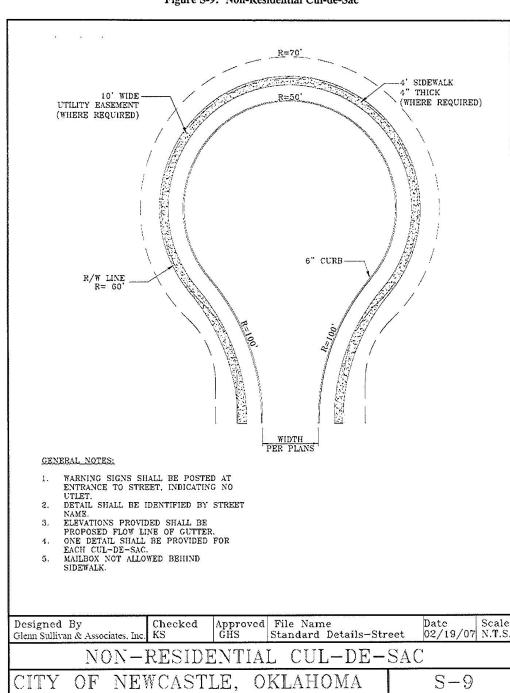


Figure S-9: Non-Residential Cul-de-Sac

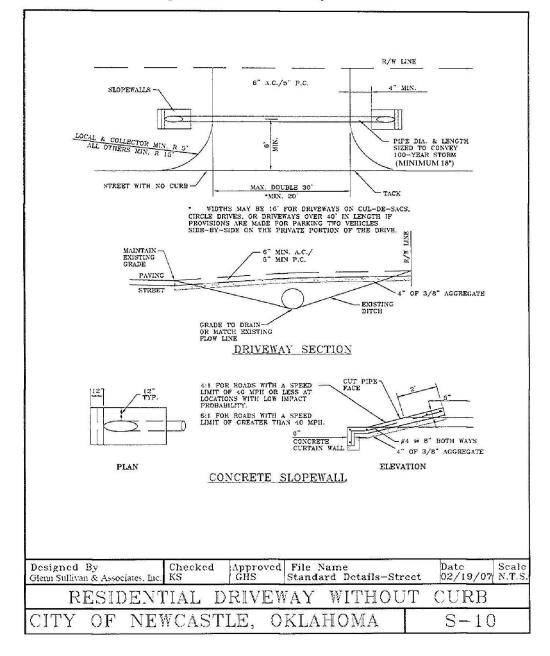


Figure S-10: Residential Driveway Without Curb

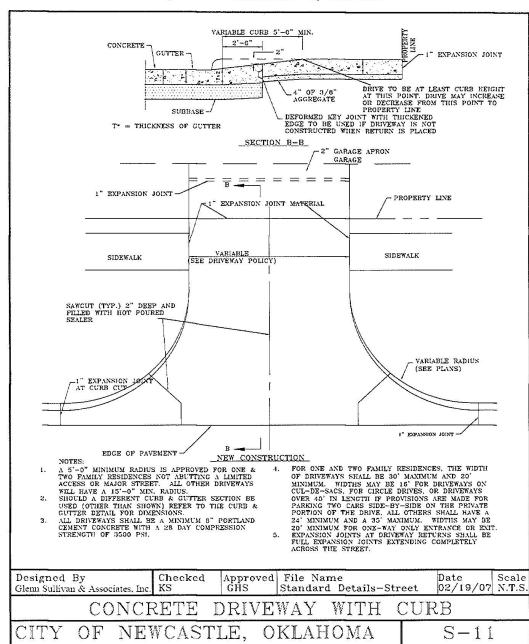
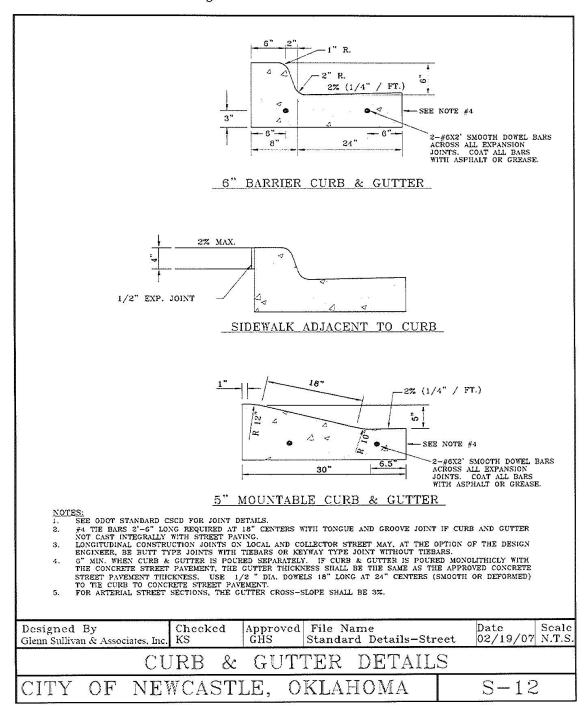


Figure S-11: Concrete Driveway With Curb

Figure S-12: Curb & Gutter Details



CORD B OR E END OF DAY'S WORK FUTURE MATCHING JOINT C OR G - D OR F FIRST STREET PAVED B OR E A -- TRANSVERSE EXPANSION JOINT B -- TIED LONGITUDINAL JOINT C -- TONGUE & GROOVE CONSTRUCTION JOINT I' MIN. D -- CONTRACTION JOINT C OR D E -- LONGITUDINAL CONSTRUCTION JOINT F -- DOWELED CONTRACTION JOINT G -- DOWELED CONSTRUCTION JOINT B OR E END OF DAY'S WORK

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> LAYOUT OKLAHOMA

Date Scale 02/19/07 N.T.S.

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Figure S-13: Joint Layout

Figure S-14: Transverse Joint Details

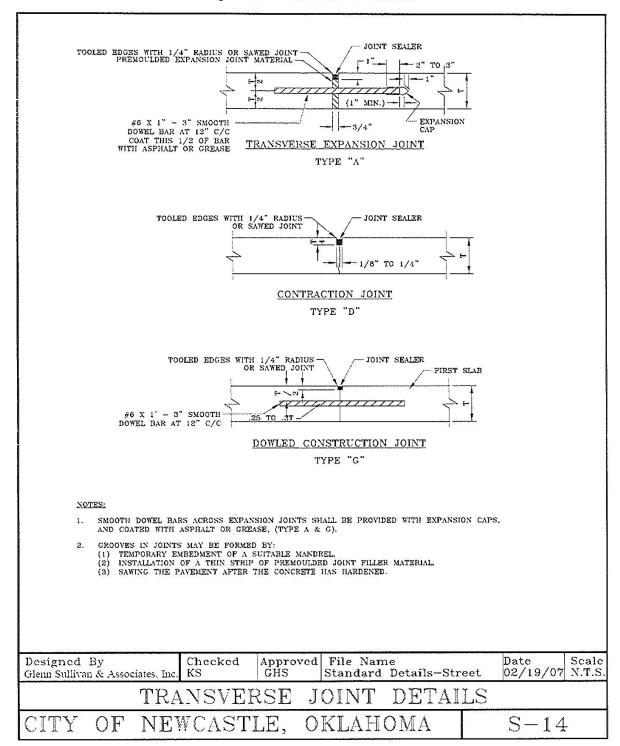


Figure S-15: Longitudinal Joint Details

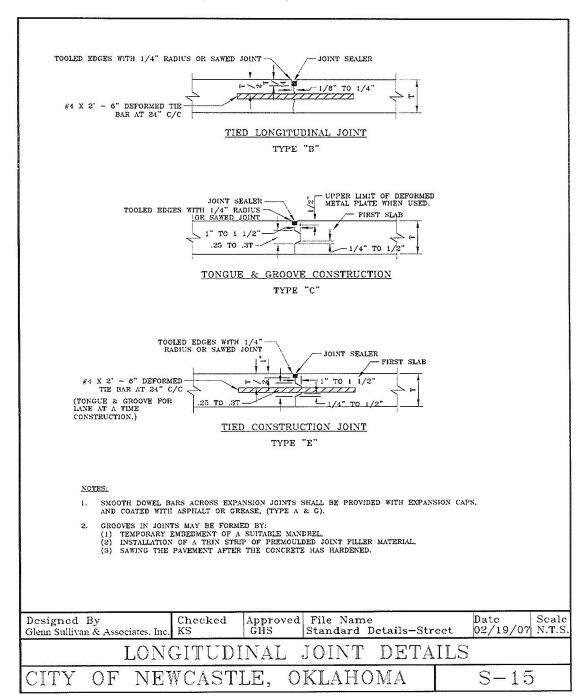
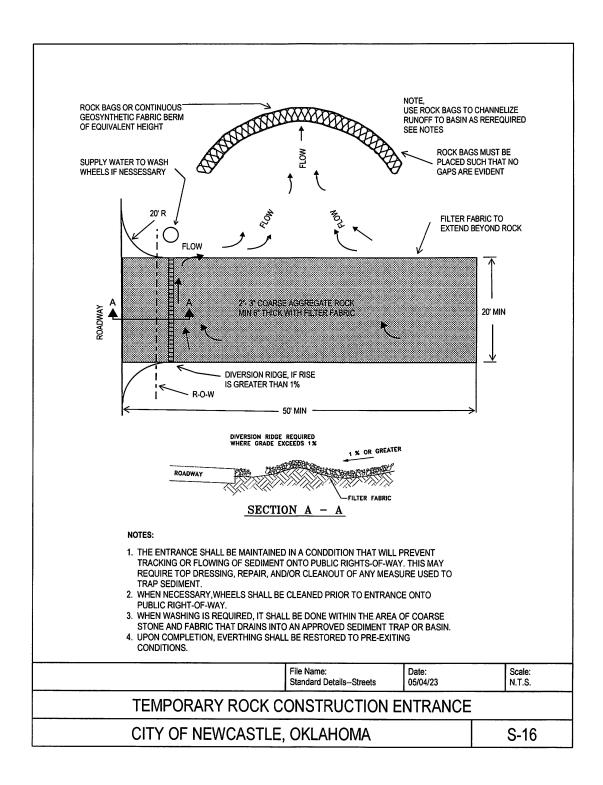


Figure S-16: Temporary Rock Construction Entrance



(Ord. 946, passed 6-12-2023)

Subdivisions 136A

Figure W-1: General Notes—Water

GENERAL NOTES

- All construction shall be in accordance with ODEQ standards and specifications, and City of Newcastle Subdivision Regulations.
- 2. PVC water pipe shall be ASTM D2241, Class 200.
- 3. HDPE water pipe shall be DR 9. Inside diameter shall be consistent with adjacent piping.
- All fire hydrants & valve boxes to be set to proposed final grade with steamer nozzle a minimum of 15" above ground level. Steamer nozzle shall face street.
- 5. Fire hydrants shall be located 5' to 9' from back of curb. Sidewalk locations to be pre-determined so that hydrants will not be located in sidewalk.
- 6. All existing water mains being abandoned by this project are to be plugged with non-shrinking, high early strength grout and shall remain the property of the owner. The cost of plugging and abandoning water mains shall be incidental if a specific bid item has not been included in the bid schedule.
- 7. When crossing streets, driveways subject to heavy traffic, alleys and structures, etc., pipe shall be installed with ¾" crusher run or river sand backfill, see standard detail. All other pipe to be installed in accordance with manufacturer's recommendations or engineer's specifications. Street crossings shall have steel casing with I.D. 4" larger than the largest O.D. of the carrier pipe. Steel casing shall extend 24" past edge of paving or back of curb.
- Set end of main stubs in cul-de-sacs at a point 5.0' off of front property line. This point being in line
 with side property line.
- In instances where flow lines are not indicated on the drawings, lines shall be constructed with a minimum of 3' cover or as directed by the field engineer.
- All staking for alignment and grade will be done under the supervision of a registered professional engineer or a registered land surveyor.
- 11. Unless specifically authorized, all gate valves are to be located at p.c. or p.t. on street curb.
- 12. All valves fittings and connections shall be installed outside proposed paving.
- 13. Disinfection of all new lines shall be in accordance with the latest revision of AWWA C651. New lines shall not be placed into service until two (2) consecutive samples taken at least 24 hours apart produce satisfactory bacteriological results.

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Figure W-2: Typical Service Line

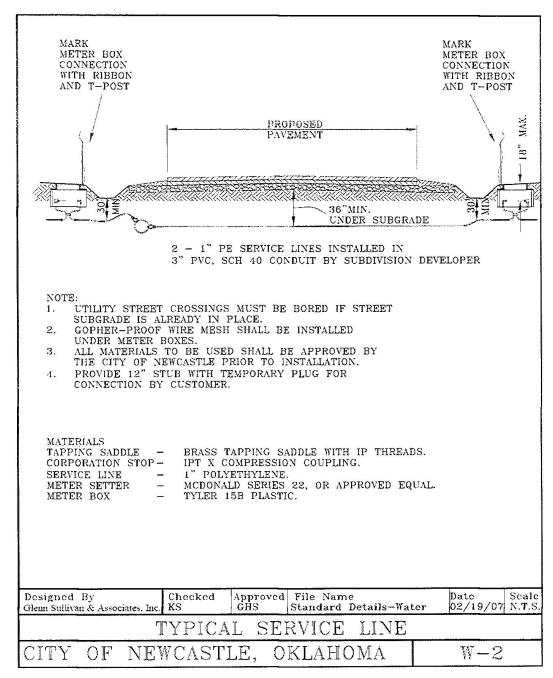


Figure W-3: Cast Iron Value Box & Lid

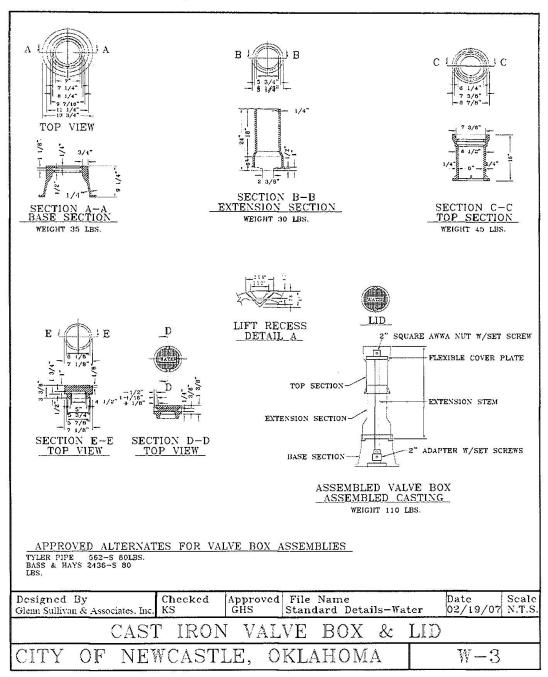
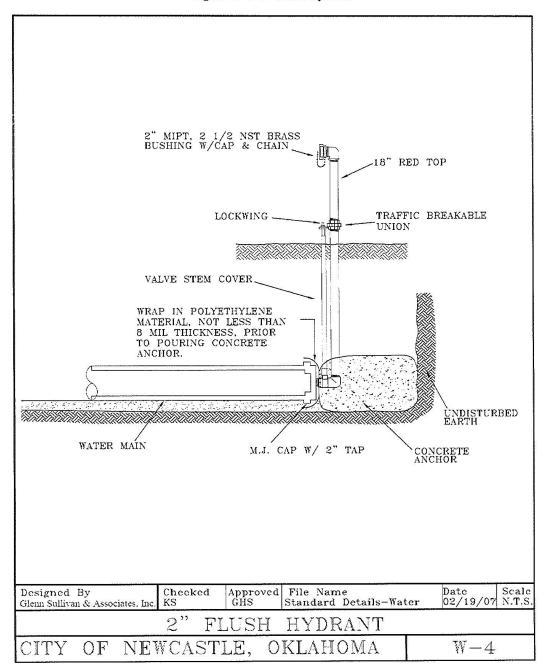


Figure W-4: 2" Flush Hydrant



Newcastle - Land Usage

Figure W-5: Thrust Blocking

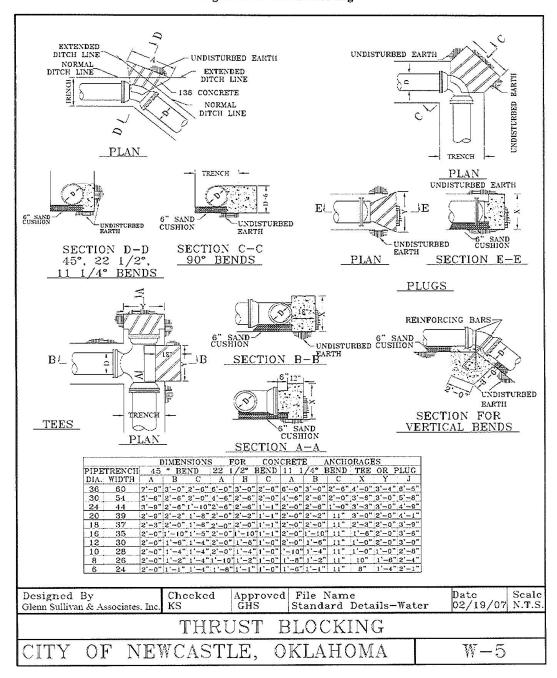


Figure W-6: Standard Road Crossing

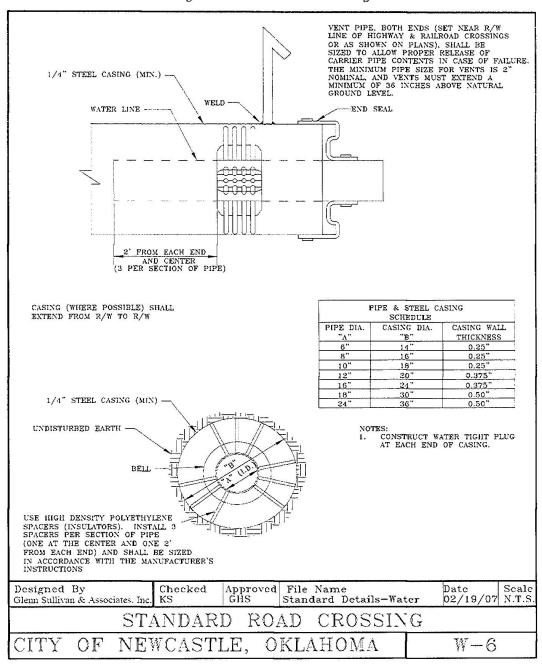
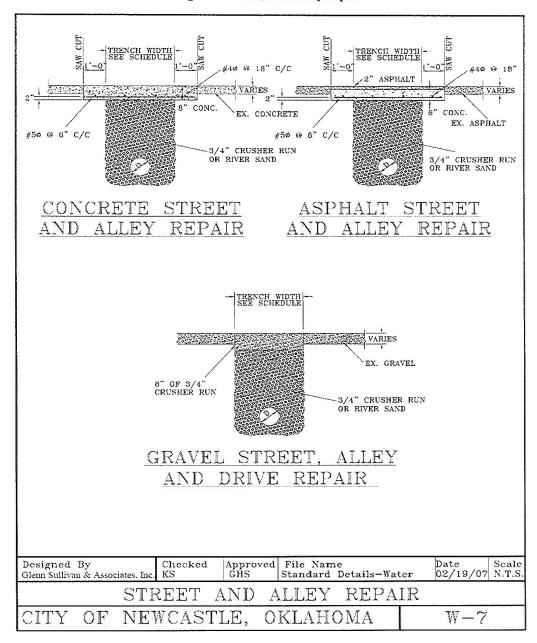


Figure W-7: Street and Alley Repair



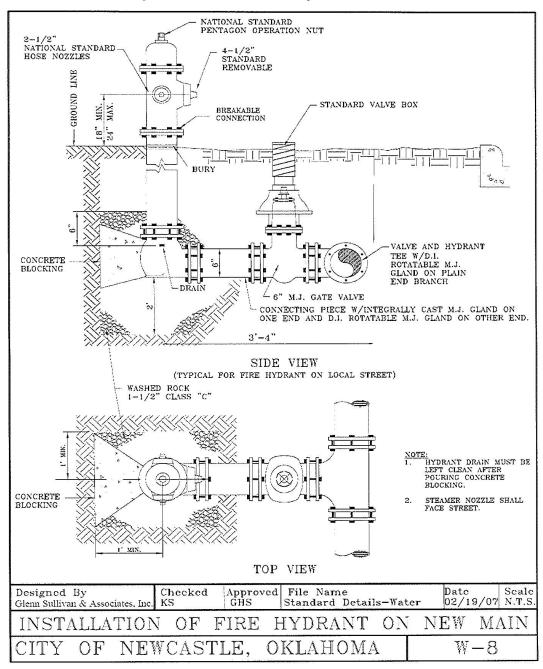
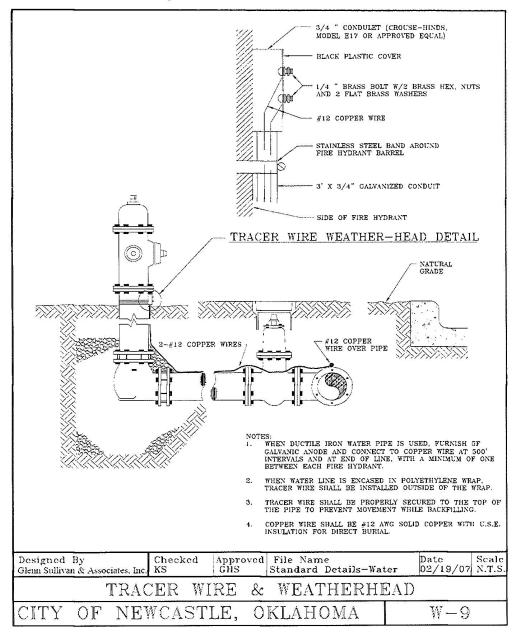


Figure W-8: Installation of Fire Hydrant on New Main

Newcastle - Land Usage

Figure W-9: Tracer Wire & Weatherhead



Sanitary Sewer

Figure SS-1: General Notes—Sanitary Sewer

GENERAL NOTES

- 1. All construction shall be in accordance with the ODEQ Standards.
- 2. The Contractor shall be responsible for the safety of all utilities, either public or private.
- Manholes shall be bid as complete and shall consist of furnishing all materials (including ring and cover) as well as testing.
- 4. Backfill around each wye connection in accordance with the Service Connection and Riser Pipe Detail.
- 5. Backfill under existing and future paving with riversand or crusher run and tamp.
- Installation shall be in accordance with ASTM D2321 "Standard Practice for Underground Installation of Thermoplastic Pipe for Sewers and Other Gravity-Flow Applications."
- 7. Water leakage inward or outward of sewer lines shall not exceed 10 gallons per 24 hours per inch diameter per mile of sewer. Water leakage tests must use at least 2-foot test head. All lines shall be tested for infiltration/exfiltration as stated in OAC 252:656-5-5(b). An air test may be used in lieu of or in addition to infiltration test on PVC sewer lines. Air testing shall be performed in accordance with Uni-Bell PVC Pipe Association Specification UNI-B-6-98, as amended "Recommended Practice for Low-Pressure Air Testing of Installed Sewer Pipe."
- 8. Deflection tests shall be performed on all pipe. The Test shall consist of pulling a mandrel through the pipe. The mandrel shall have a diameter equal to 95% of the inside diameter of the pipe. The test shall be performed without mechanical pulling devices. The allowable deflection shall not exceed five (5) percent of the internal diameter of the pipe as stated in OAC 252:656-5-5(a).
- 9. Manholes and other watertight structures shall be tested before backfilling by filling with water to overflowing, or other level as directed by the City, and observing the water surface level twenty-four hours thereafter. Exterior surfaces shall be examined for leaks or leakage. Leakage is within allowable limits for structures when there is no visible sign of leakage and where the water surface does not drop more than 1-inch during the twenty-four hour leakage test. Dampness on the exterior wall surface during the test period will not be considered leakage, except in the case of precast concrete structures.
- 10. In lieu of the hydrostatic testing described above, the contractor may vacuum test the manholes in accordance with ASTM C1244-02 "Standard Test Method for Concrete Sewer Manholes by the Negative Air Pressure (Vacuum) Test Prior to Backfill."

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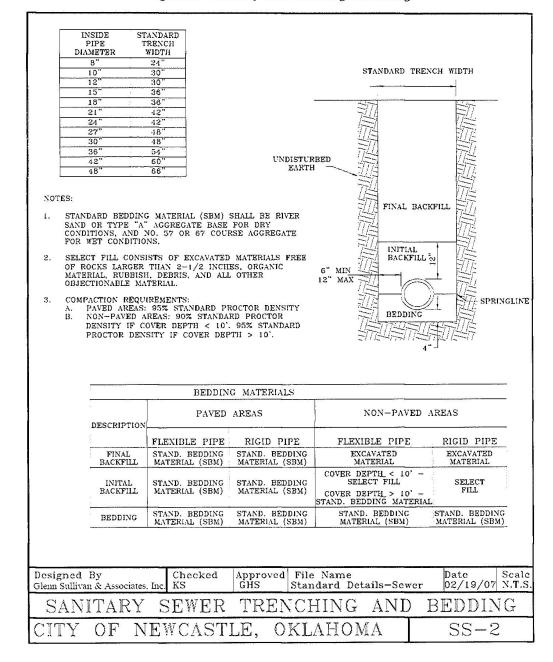


Figure SS-2: Sanitary Sewer Trenching and Bedding

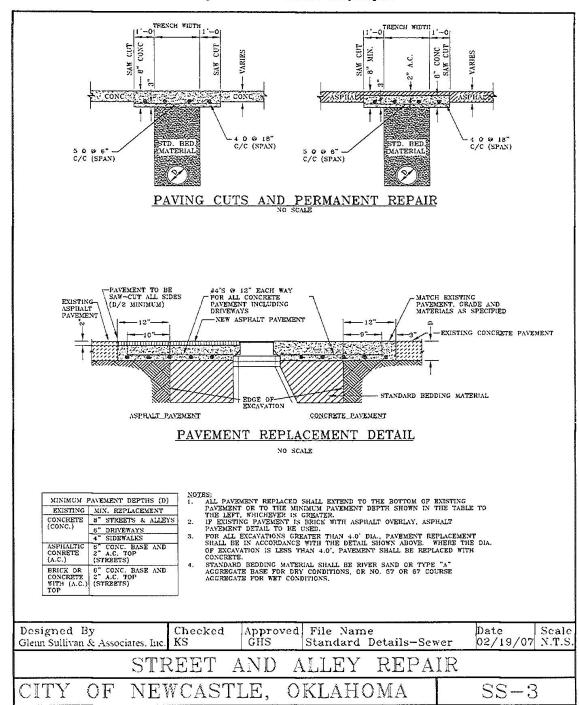


Figure SS-3: Street and Alley Repair

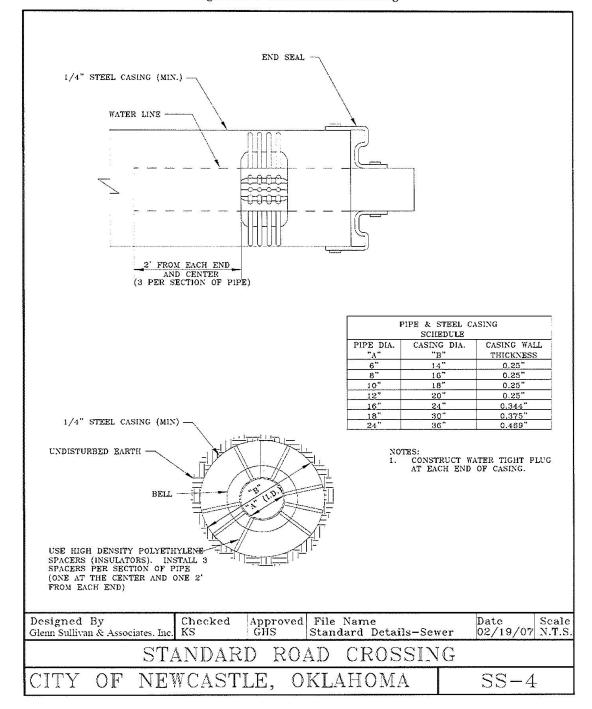


Figure SS-4: Standard Road Crossing

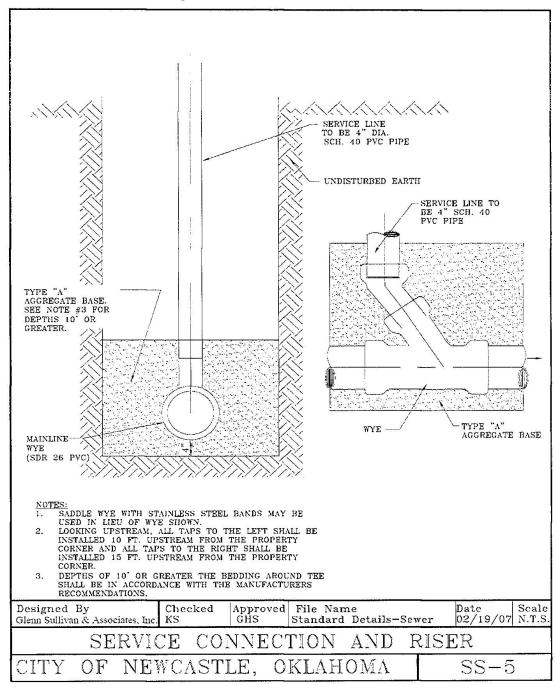
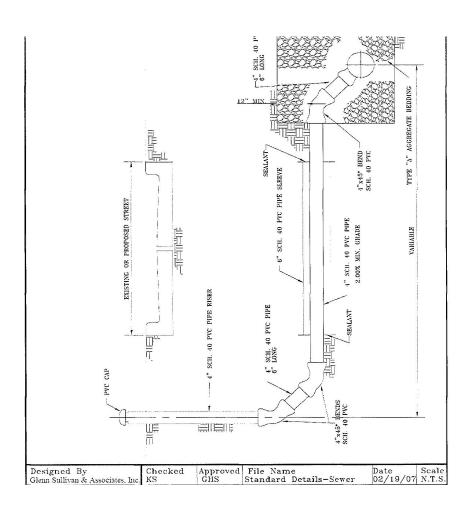
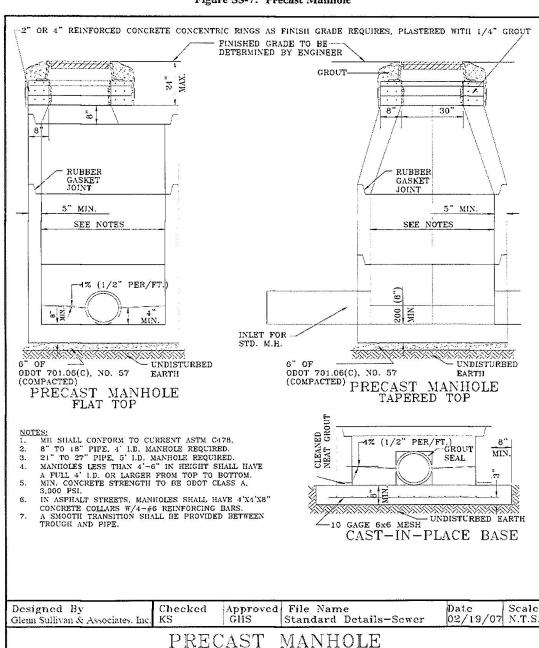


Figure SS-5: Service Connection and Riser





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Figure SS-7: Precast Manhole

Figure SS-8: Cast in Place Manhole

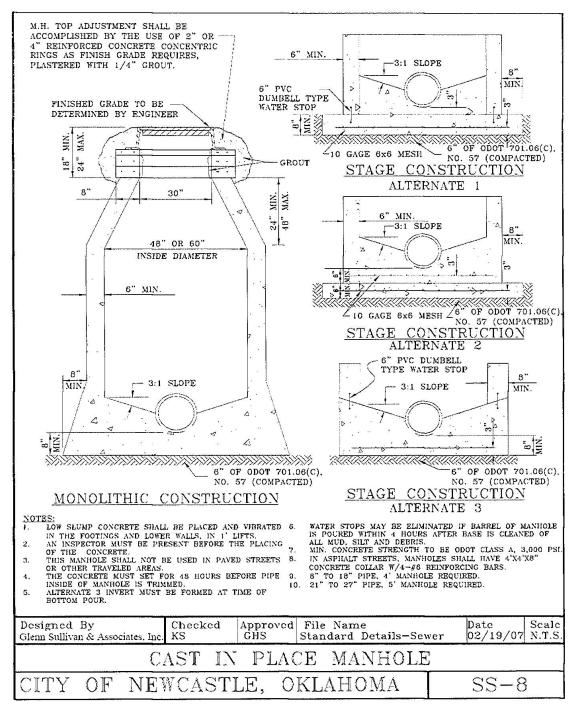
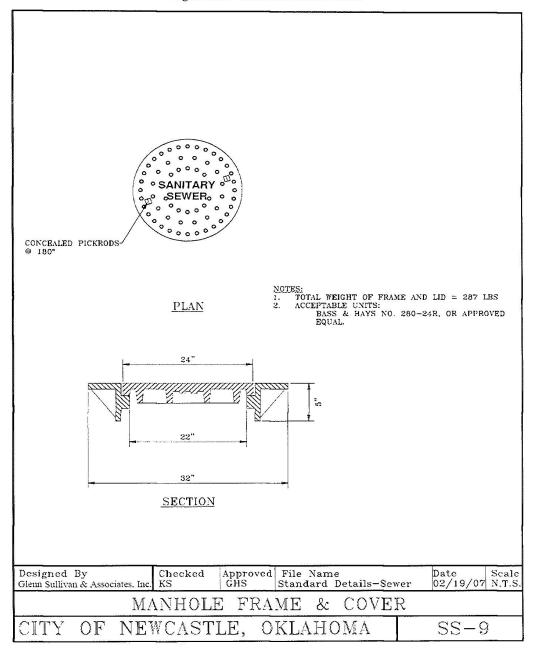
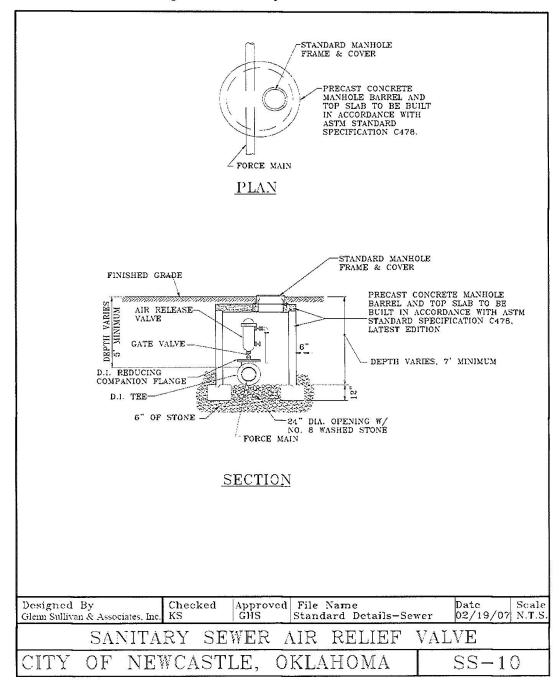


Figure SS-9: Manhole Frame & Cover



Newcastle - Land Usage

Figure SS-10: Sanitary Sewer Air Relief Valve



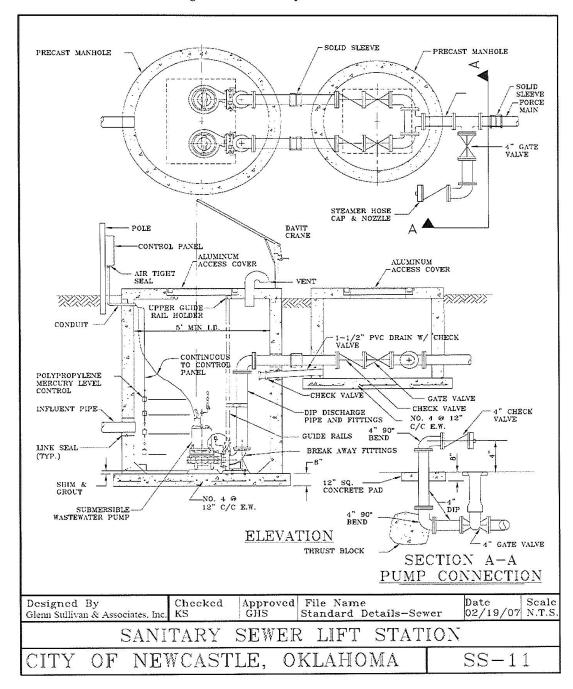
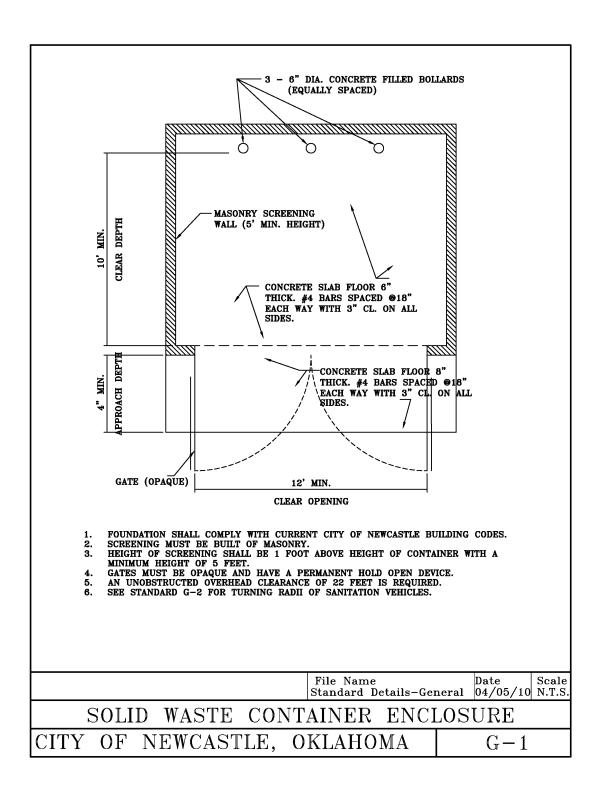
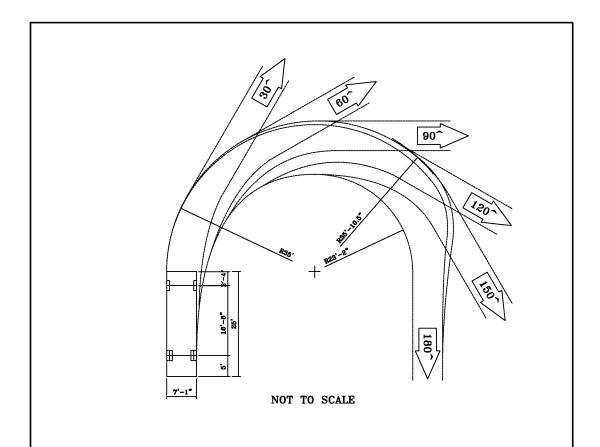


Figure SS-11: Sanitary Sewer Lift Station



Subdivisions 156A



FROM THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS; "A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS"; 1990: SINGLE UNIT TRUCK DESIGN VEHICLE

	File Name Standard Details-General	Date 04/05/10	Scale N.T.S.
SANITATION TRUCK	TURNING RAI	DIUS	
CITY OF NEWCASTLE, O	KLAHOMA	G-2	

